

M.M = 15

Time = 1 hour

Date 15/1/18

10.30 - 11.30

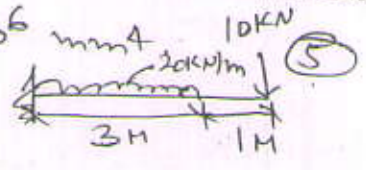
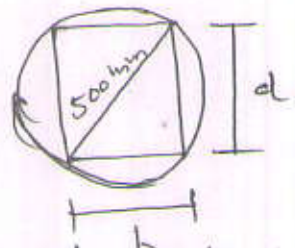
S.M

Solve All Questions, Carry Equal Marks

1. Calculate the Dimension of the Strongest Rectangular beam that can be cut - out of Log of a wood 500 mm in dia. (5)
2. Calculate Shear stress at Important points of T-section width 80mm, Total depth 130mm, Thickness of flange 10mm, Thickness of web 10mm subjected to Shear force 10kN. Draw the shear stress diagram Also (5)
3. Calculate the Deflection at free end of Cantilever Beam shown in figure, $E = 2 \times 10^5 \text{ N/mm}^2$ and $I = 250 \times 10^6 \text{ mm}^4$ (5)

Solution: →

1. $b =$ width of beam
 $d =$ depth of beam



for least wastage Log of wood b to be carved out should have diagonal 500mm

$$500^2 = b^2 + d^2$$

Section Modulus $Z = \frac{bd^2}{6}$

$$d^2 = 500^2 - b^2$$

$$\frac{b(250000 - b^2)}{6} = \frac{250000b - b^3}{6}$$

The Beam to be Strongest the Z must be maximum. Hence Z : Maximum

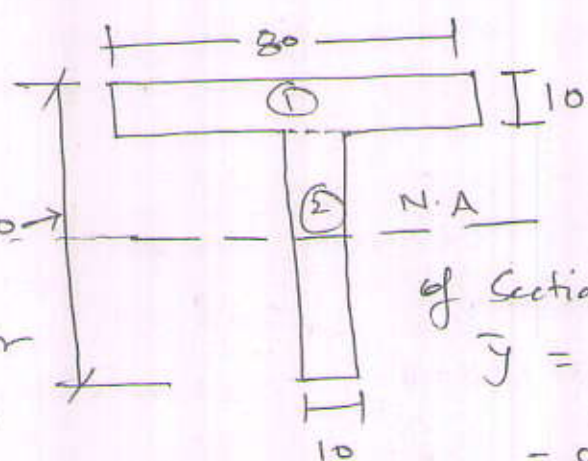
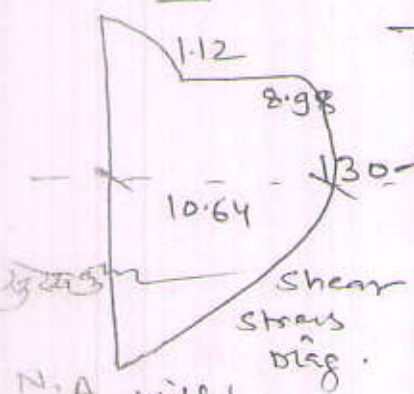
$$\frac{250000 - 3b^2}{6} = 0$$

$$\frac{dz}{db} = 0 \quad \frac{dz}{db} = \frac{d}{db} \left(\frac{250000b - b^3}{6} \right) = 0$$

$$b = 288.7 \text{ mm}, \quad d = 408.20 \text{ mm}$$

Sol:

Z



Shear force $V = 10 \text{ kN}$, 10000 N

let $\bar{y} =$ distance of c.g of Section from the top of flange.

$$\bar{y} = \frac{a_1 y_1 + a_2 y_2}{a_1 + a_2}$$

$$= \frac{80 \times 10 \times 5 + 120 \times 10 \times 70}{80 + 120} = 44 \text{ mm}$$

N.A will be at a distance of 44 mm from top of flange

M.O.I of the total Section about N.A

$$I = \frac{80 \times 10^3}{12} + 80 \times 10 \times (44 - 5)^2 + \frac{10 \times 120^3}{12} + 120 \times 10 \times (70 - 44)^2$$

$$I = 3474667 \text{ mm}^4$$

(i) Shear Stress at top of flange = 0

(ii) Shear Stress in the flange at the Junction with web

$$A = 80 \times 10 = 800 \text{ mm}^2 \quad \bar{y} = 44 - \frac{10}{2} = 39 \text{ mm}$$

$$b = 80 \text{ mm}$$

$$\tau = \frac{V A \bar{y}}{I b} = \frac{10,000 \times 800 \times 39}{3474667 \times 80} = 1.12 \text{ N/mm}^2$$

(iii) Shear Stress in web $A = 800 \text{ mm}^2, \bar{y} = 44 - \frac{10}{2} = 39 \text{ mm}$

$$b = 10 \text{ mm}$$

$$\tau = \frac{V A \bar{y}}{I b} = \frac{10000 \times 800 \times 39}{3474667 \times 10} = 8.98 \text{ N/mm}^2$$

(iv) Shear Stress at N.A

$$A \bar{y} = 80 \times 10 \times 39 + 10 \times (44 - 10) \times \left(\frac{44 - 10}{2} \right) = 36980 \text{ mm}^2$$

$$b = 10 \text{ mm}$$

$$\tau = \frac{10000 \times 36980}{3474667 \times 10} = 10.64 \text{ N/mm}^2$$

(v) Shear Stress at Bottom of Web is zero. Diagram is shown with Section.

Sol. 3 The Max. Deflection at the free end.

$$I = \text{Deflection due to Point load} = \frac{W l^3}{3EI} = \frac{10,000 \times (4000)^3}{3 \times 2 \times 10^5 \times 2.50 \times 10^6} \Rightarrow 4.27$$

$$II = \text{Deflection due to U.D.L} \Rightarrow \frac{w a^4}{8EI} + \frac{w a^3}{6EI} (l - a)$$

$$\frac{20 \times (3000)^4}{8 \times 2 \times 10^5 \times 2.50 \times 10^6} + \frac{20 \times (3000)^3}{6 \times 2 \times 10^5 \times 2.50 \times 10^6} (4000 - 3000) \Rightarrow 5.85$$

Max. Deflection at the free end is = 4.27 + 5.85 = 10.12 mm

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LCE 18-1-18